



Transportation of a water-based slurry in an open furrow launder or stream

by J.P.F. Charpin*, T.G. Myers*, M. Lombe*, and P. de Hill†

Synopsis

The transport of large boulders in a furrow from a mining area to a nearby pond was considered. The furrow is filled with a mixture of water and soil particles flowing down to the pond at high velocity. Due to operating constraints, the slope of the furrow is reduced progressively. A relationship is derived, relating the slope of the furrow and the composition of the fluid to the maximum size and shape of the transported boulders. The characteristics of the boulders carried all the way down to the pond may then be determined.

Introduction

A mineral processing plant produces vast amounts of waste material. This is made up of rocks of all sizes, ranging from very fine pebbles, with a diameter less than a millimetre, to rocks, with a diameter up to twenty centimetres. In the set-up presented at the study group, the waste is washed away with high pressure water and flows downhill to a pond three kilometres away and approximately three hundred metres lower. As it flows, the slurry carves a channel, typically one metre by one metre. Since the mining area, where the material is produced, is slowly moved away from the pond, the average angle of the furrow to the horizontal decreases progressively. As the angle decreases the possibility of larger rocks becoming stuck in the channel increases.

Every month, about one million tons of material is produced, most of it consisting of particles with a diameter less than a millimetre. They are washed away by water at the ratio 1000 kg of water for 200 kg to 400 kg of material. Considering the size of the channel created by the flow, the flux of material leads to an average mixture velocity in the channel between $v = 1 \text{ m}\cdot\text{s}^{-1}$ and $v = 2 \text{ m}\cdot\text{s}^{-1}$. In these conditions, the flow is clearly turbulent, the Reynolds number is well above $Re = 10^6$.

Due to the high velocity, small particles should be carried away easily by the flow. This study therefore focuses on the largest boulders, with a diameter that may reach up to 20 cm. In the following section, we determine a criterion for boulders to be washed away. The results depend on the Shields number, the ratio of shear stress and gravity, the two main forces applied on the boulder and the shape of the boulder. Numerical results are then presented.

Moving large boulders

Figure 1 shows a typical configuration for the transport of large boulders. The channel is inclined at an angle θ to the horizontal and the boulder lies at the bottom of it. The potential movement of the boulder is governed by two main forces:

- ▶ Shear stress and pressure is applied on the upwards facing surfaces and pushes the boulder in the downstream direction
- ▶ Effective gravity, the difference between gravity and the Archimedes force.

The ratio between these two forces is summarized in a non-dimensional parameter called the Shields number:

$$S = \frac{\tau}{\Delta\rho g D} \quad [1]$$

where:

τ is the shear stress applied by the flow on the boulder,
 $\Delta\rho = \rho_b - \rho_f$ is the difference between the density of the boulder, ρ_b , and the density of the fluid, ρ_f ,
 g denotes gravity,

* University of Cape Town, South Africa.

† Lynx Geosystems.

© The Southern African Institute of Mining and Metallurgy, 2007. SA ISSN 0038-223X/3.00 + 0.00. Paper received Apr. 2006; revised paper received Jan 2007.

Transportation of a water-based slurry in an open furrow launder or stream

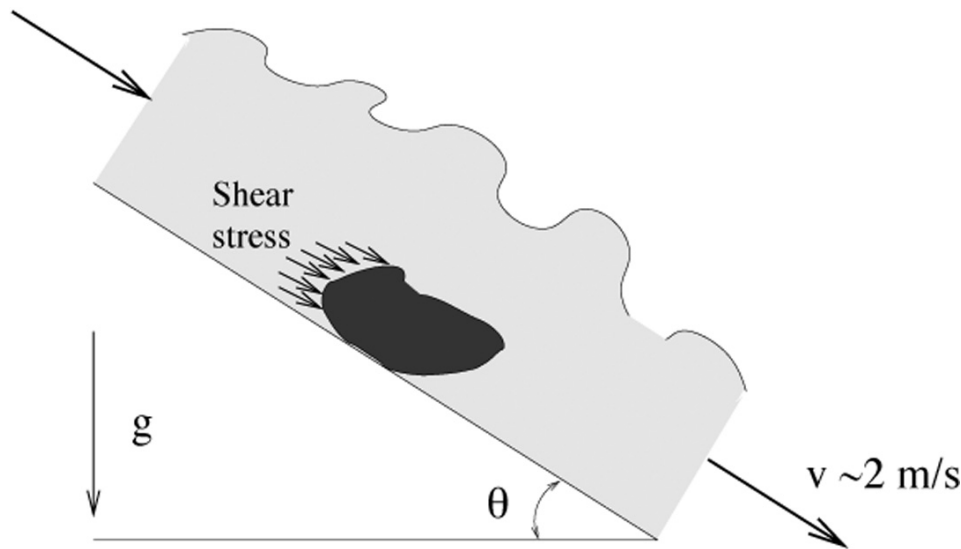


Figure 1—Typical configuration for large boulders

D is the effective diameter of the boulder. (Note for non-spherical boulders we must take some approximate diameter, for example by considering a sphere of equal volume.)

Critical values of the Shields number may be found in tables. Below the critical value, gravity dominates and the boulder will remain motionless on the ground. When the Shields number exceeds the critical value, shear stress is the dominant force and movement is triggered. To calculate the minimum angle to the horizontal required to wash away boulders, the shear stress in Equation [1] will be expressed as a function of the flow characteristics, geometry of the channel, slope, particle concentration, density and dynamic viscosity, and the resulting expression will be combined with the critical value of the shear stress.

Expression for the Shields number

The average velocity of the particle and water mixture is such that the flow is highly turbulent. In this case, an appropriate expression for the shear stress applied to the boulder, τ , is, (Patel 1999):

$$\tau = \frac{f}{8} \rho_f u^2, \quad [2]$$

where

f is the friction factor. For the type of turbulent flow considered in this study, according to Patel (1999), the friction factor may be expressed as:

$$f = \frac{0.292}{Re^{1/4}} \text{ where } Re = \frac{\rho_f u L}{\mu_f} \quad [3]$$

and Re is the Reynolds Number, L is a typical length of the flow, ρ_f and μ_f are the density and dynamic (or shear) viscosity of the mixture respectively. According to experimental observations, $L = 1$ m is appropriate here. The density and dynamic viscosity vary significantly with the concentration of soil particles in the fluid mixture. The

density of the mixture increases with particle volumetric concentration, c :

$$\rho_f = \rho_w + c(\rho_b - \rho_w), \quad [4]$$

where ρ_w and ρ_b denote the density of pure water and soil respectively (Abraham *et al.* 2001 and Guy *et al.* 1990). The dynamic viscosity may be approximated by:

$$\mu_f = \mu_w \left(1 + \frac{0.75c}{0.605 - c} \right)^2, \quad [5]$$

where μ_w is the dynamic viscosity of pure water (see Coussot 1997). Equation [5] is valid up to $c = 0.45$. The average soil volumetric concentration in the fluid may be estimated around $c \approx 0.1$, therefore Equation [5] is entirely appropriate here.

u is the velocity of the fluid. According to experimental observations, this may be estimated around $2 \text{ m}\cdot\text{s}^{-1}$. The velocity may be related to the slope of the channel using, for example, the Glauker-Manning formula (Chanson 1999), valid for fully rough turbulent flows:

$$u = \frac{1}{n} R_H^{2/3} \sqrt{\sin \theta}, \quad [6]$$

where

n is the Manning coefficient. A typical value for earth surface roughness is $n = 0.025$. For gravels, the value is $n = 0.029$. The value for this parameter will therefore be taken as $n = 0.027$.

R_H denotes the hydraulic radius,

θ is the slope of the channel.

Combining Equations [2–6], the right-hand side of Equation [1] may be rewritten:

$$S = \frac{0.292 (\rho_w + c(\rho_b - \rho_w))^{3/4}}{8gD (\rho_b - \rho_w)(1 - c)} \times \left(\frac{\mu_w}{L} \right)^{1/4} \sqrt{1 + \frac{0.75c}{0.605 - c}} R_H^{7/3} \frac{(\sin \theta)^{7/8}}{n^{7/4}}. \quad [7]$$

Transportation of a water-based slurry in an open furrow launder or stream

The slope of the channel may then be expressed as:

$$\theta = \arcsin \left[\frac{n^2}{R_H^{4/3}} \left(\frac{8gDS}{0.292} (\rho_b - \rho_w)(1 - c) \right)^{8/7} \times \left(\frac{1}{(\rho_w + c(\rho_b - \rho_w))^3} \right)^{2/7} \times \left(\frac{L}{\mu_w \sqrt{1 + 0.75c/(0.605 - c)}} \right)^{2/7} \right] \quad [8]$$

The Shields number, S , remains the only unknown quantity on the right hand side of Equation [8]: the critical value of this parameter is required to determine when large boulders will move. It depends on the boulder, flow and fluid characteristics. This will now be detailed.

Critical Shields number

The value of the critical Shields number corresponding to the present setting may be found in Julien (1994):

$$S_c = 0.06 \tan \phi \quad [9]$$

This holds when the nondimensional particle parameter, d_* , satisfies:

$$d_* = D \sqrt[3]{\frac{\rho_f(\rho_b - \rho_f)g}{\mu_f^2}} > 50$$

The angle ϕ is called the angle of repose. This is the critical angle of the channel floor at which a particle would move of its own accord, i.e. with no fluid motion (Julien 1994). The angle of repose is zero for a sphere on its own and increases with the angularity of the boulder. Equation [9] is valid if the bottom of the channel is entirely covered with large boulders of diameter D . In the present situation the channel is most likely composed of smaller particles, in which case Equation [9] will overestimate the critical Shields number.

The minimum slope of the channel required to transport large boulders may then be expressed as a function of the angle of repose and Equation [8] becomes:

$$\theta = \arcsin \left[\frac{n^2}{R_H^{4/3}} (1.64gD \tan \phi_s)^{8/7} \times ((\rho_b - \rho_w)(1 - c))^{8/7} \times \left(\frac{1}{(\rho_w + c(\rho_b - \rho_w))^3} \right)^{2/7} \times \left(\frac{L}{\mu_w \sqrt{1 + 0.75c/(0.605 - c)}} \right)^{2/7} \right] \quad [10]$$

This shows that the angle of the slope, θ , required to move a large boulder increases with the angle of repose, ϕ and the boulder diameter, D . This is confirmed by the observations at the end of the channel (and common sense). A straightforward analysis of the relationship shows that the angle θ decreases when the volumetric particle concentration in the flow increases. Note the volumetric concentration, c , is the only parameter that may be adjusted in practice. A more quantitative analysis of the relationship will now be performed.

Numerical results

Figure 2 shows the angle of the slope necessary to move boulders with a diameter varying up to $d = 20\text{cm}$ and angle of repose up to $\phi = 50^\circ$. As could be expected, the required angle of the slope for small particles is extremely low, whatever their angularity. When the diameter of the boulder increases, the effect of the angle of repose appears more clearly: low values of this parameter (corresponding to almost spherical boulders) do not require a high channel inclination. The more angular the boulder gets, i.e. the larger the angle of repose, the steeper the slope of the channel required to move it.

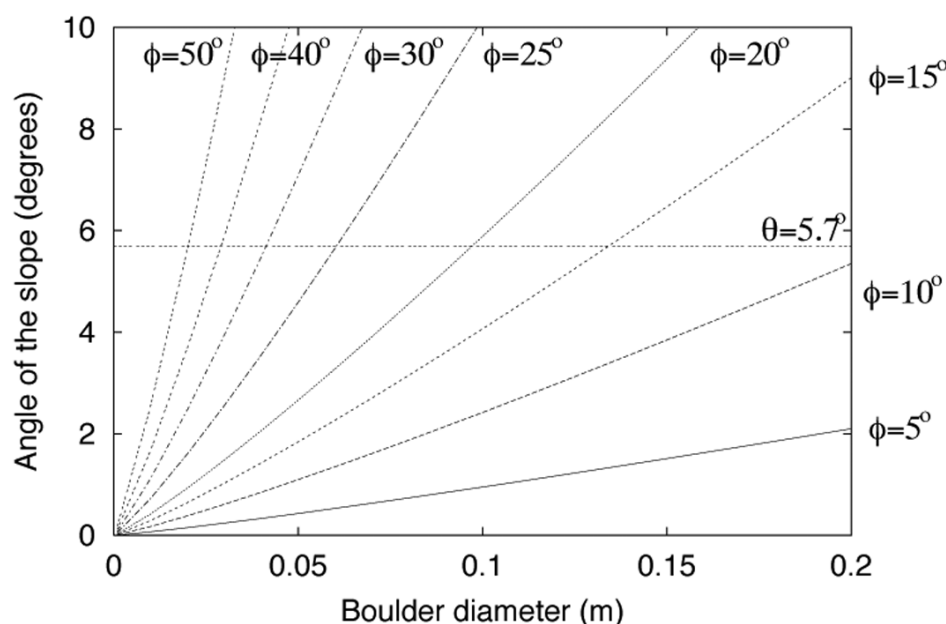


Figure 2—Slope angle, θ , required to move a boulder as a function of its diameter and angle of repose, ϕ , in a slurry with the particle volumetric concentration $c = 0.1$

Transportation of a water-based slurry in an open furrow launder or stream

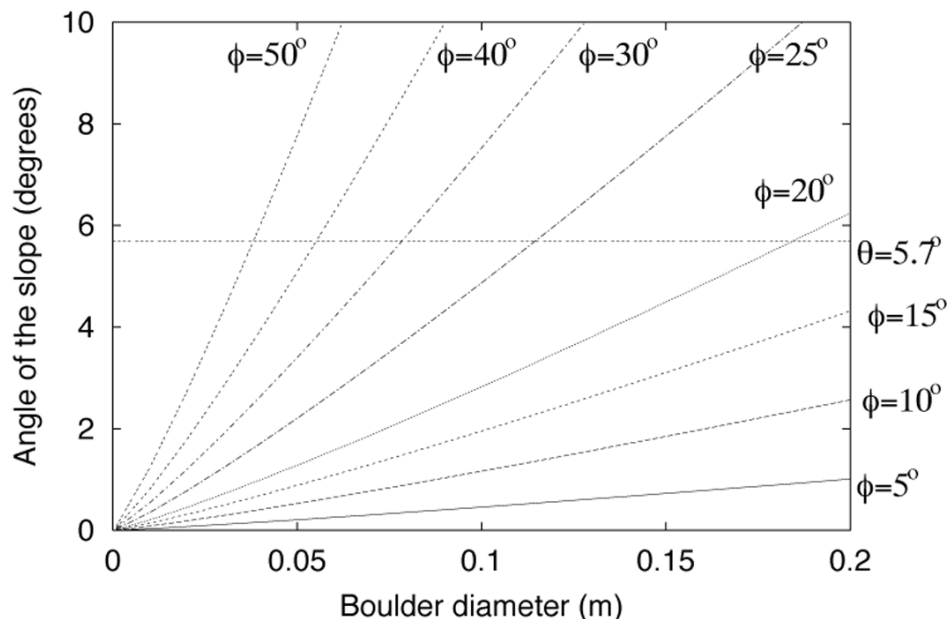


Figure 3—Slope angle, θ , required to move a boulder as a function of its diameter and angle of repose, ϕ , in a slurry with the particle volumetric concentration $c = 0.3$

In the configuration presented at the study group, the channel is 3 kilometres long and the decrease in altitude is 300 metres. The corresponding average angle of the slope is therefore $\theta_0 = 5.7^\circ$. In this configuration with a volumetric concentration $c \approx 0.1$, boulders with a diameter $d = 20$ cm may be transported only if their angle of repose is lower than $\phi \approx 11^\circ$ i.e. they must be close to spherical but again, the critical value of the shear stress is overestimated and real values could be significantly higher. If the diameter is $d = 10$ cm, the allowed angle of repose increases to $\phi \approx 18^\circ$. However, due to the operating conditions, the volumetric concentration of particles in the fluid may vary significantly: the waste is piled up at the top of the hill and a much larger quantity of material may suddenly be washed away, triggering movement for more angular boulders. For the channel slope $\theta_0 = 5.7^\circ$ and volumetric concentration $c = 0.3$, a 20 centimetre diameter boulder may have an angle of repose up to $\phi \approx 16^\circ$. For a 10 centimetre diameter boulder, the angle increases to $\phi \approx 26^\circ$ and the boulder will still be washed away. These values may be read from Figure 3.

Once movement has been triggered, the boulder's inertia allows it to keep on rolling down the hill, even if the starting conditions are no longer satisfied. The boulder may then either roll down to the pond or settle at the bottom of the channel until movement conditions are once again met by the varying volumetric concentration.

Practically, the model states that boulders with low angularity may reach the pond. This agrees with the experimental observations: the largest boulders reaching the pond are close to spherical.

Conclusion

Turbulent flow in an open channel was studied to derive a formula relating the slope of a channel to the characteristics of the boulders the flow may wash away. This study is based on the critical value of the Shields number, which determines

when the boulders to be washed away will start rolling down the channel. The study did not focus on the later course of the boulders; the condition derived here indicates only when movement is triggered. Inertia then allows boulders to continue moving even when the criterion is not satisfied. A geometrical study of the channel shape could complete the study and ensure that the boulders roll as far as possible.

Acknowledgements

The contributions of A. Fitt, University of Southampton, UK, N. Fowkes, University of Western Australia, South Africa, K. Pereira and R. Sarracino, both from the University of Witwatersrand, Johannesburg, South Africa, are here kindly acknowledged. All authors would like to thank Mr. P. de Hill from Lynx Geosystems for introducing the problem and assisting in answering questions. T.G. Myers acknowledges support of this work under the National Research Foundation of South Africa grant number 2053289. J.P.F. Charpin acknowledges the support of the Claude Harris Foundation.

References

- ABRAHAM, A.D., LI, G., KRISHNAN, C., and ATKINSON, J.F. A sediment transport equation for interrill overland flow on rough surfaces, *Earth Surface Processes and Landforms*, vol. 26, 2001. pp. 1443–1459.
- CHANSON, H. *The Hydraulics of Open Channel Flow*, Chapter 4, Wiley, New York. 1999.
- COUSSOT, P. *Mudflow Rheology and Dynamics*, Chapter 3, Balkema, Rotterdam. 1997.
- GUY, B.T., DICKINSON, W.T., RUDRA, R.P., and WALL, G.J. Hydraulics of sediment-laden sheetflow and the influence of simulated rainfall, *Earth Surface Processes and Landforms*, 1990. vol. 15, pp. 101–118.
- JULIEN, P.Y. *Erosion and Sedimentation*, Chapter 7, Cambridge University Press, Cambridge. 1994.
- PATEL, B.R. Internal Flows. *Fundamentals of Fluid Mechanics*, Chapter 5.2, J.A. Schetz and A.E. Fuhs, (ed.), John Wiley & Sons, Inc. 1999. ◆